

916395**Textured geomembrane for the staged raising of Cero do Lobo dam**

Cambridge, M; Maranhão de Neves, E

Int J Water Power Dam Constr V43, N6, June 1991, P57-64

The Neves Corvo mine in southern Portugal will require a 39m high embankment dam for tailings storage. The initial phase used a conventional clay cored structure using locally won materials. Shortage of additional suitable clay has led to specification of a textured HDPE geomembrane for subsequent stages. Laboratory tests were used to develop suitable mixtures for low permeability seeds and damage prevention measures. Design of anchorage connections for the membrane with both the existing clay core and the foundation at the abutments is described.

916396**Upstream dam construction: an instrumented test fill evaluation**

Gale, S M

Proc 4th International Conference on Geotextiles, Geomembranes and Related Products, The Hague, 28 May-1 June 1990 V1, P9-15. Publ Rotterdam: A A Balkema, 1990

Test fills were evaluated during design of a 5km long, 15m high retention dam upstream on a 14m deep deposit of tailings from a taconite mine. Unreinforced, geotextile reinforced, and geogrid reinforced, comprehensively instrumented sections were built, and fill performance and soil-reinforcement interaction studied. Strain compatibility between fill and reinforcement, strain anisotropy of the reinforcement, and time dependence of strain are discussed. Limit equilibrium analysis is used to describe the behaviour. Geotextile along the tailings-fill interface was used for the full scale structure.

916397**Performance of a geogrid in a cohesive soil test embankment**

Sego, D C; Scott, J D; Richards, E A; Liu, Y

Proc 4th International Conference on Geotextiles, Geomembranes and Related Products, The Hague, 28 May-1 June 1990 V1, P67-72. Publ Rotterdam: A A Balkema, 1990

A 12m high embankment was constructed using a silty clay. One section was unreinforced, three sections contained different geogrid reinforcements. Geometry, reinforcement layout, soil properties, and instrumentation are described. The three year construction record is presented. Performance of one section (Tensar geogrids) over this period and for one year after completion is described. Horizontal and vertical displacements and grid strains are reported. Further analysis will examine interactions between reinforcement and fill.

916398**Experimental study on prototype polymer grid reinforced retaining wall**

Kotera, K; Miki, H; Kudoh, K; Nakamura, K; Minami, T; Iwasaki, K; Nishimura, J; Fukuda, N; Taki, M

Proc 4th International Conference on Geotextiles, Geomembranes and Related Products, The Hague, 28 May-1 June 1990 V1, P73-78. Publ Rotterdam: A A Balkema, 1990

A steep, 6m high embankment and a retaining wall were constructed using cohesive silty sand reinforced by geogrids. Loads were applied by banking and the water level was suddenly lowered after subsidence. Deformation and settlement of the structures and strain of the reinforcement were monitored. Stability analyses by the rotational slip and two wedge limit equilibrium methods were not appropriate to describe the behaviour of the cohesive fill, and improved design methods are needed.

916399**Alleviating bridge approach settlement with geosynthetic reinforcement**

Wu, J T H; Helwany, H B

Proc 4th International Conference on Geotextiles, Geomembranes and Related Products, The Hague, 28 May-1 June 1990 V1, P107-111. Publ Rotterdam: A A Balkema, 1990

Excessive differential settlements between bridge abutments and the backfill of the bridge approach are frequently seen. Reinforcement of the fill with geosynthetics is often used to try to minimise these settlements. The mechanisms and effectiveness of this technique have been examined using finite element analysis. Influence of reinforcement stiffness and properties of the foundation soil on settlement is investigated for cases where the wall is separated from the reinforced fill by a thin, collapsible layer and where the reinforcement is attached directly to the abutment wall.

916400**Implications of adopting an allowable geosynthetic strain in estimating stability**

Rowe, R K; Mylleville, B L J

Proc 4th International Conference on Geotextiles, Geomembranes and Related Products, The Hague, 28 May-1 June 1990 V1, P131-136. Publ Rotterdam: A A Balkema, 1990

A layer of geosynthetic between foundation and fill is commonly used to improve the performance of embankments on soft foundations. Some implications of adopting limits on reinforcement strain in design are examined by comparing results of finite element stability analyses with those of the limit equilibrium method. Potential problems in the case of strain softening soils are investigated. The case of substitution of two layers of reinforcement of a given modulus for one with twice that modulus is examined.

916401**Parametric studies of the behaviour of a reinforced embankment**

Hird, C C; Kwok, C M

Proc 4th International Conference on Geotextiles, Geomembranes and Related Products, The Hague, 28 May-1 June 1990 V1, P137-142. Publ Rotterdam: A A Balkema, 1990

Finite element modelling of an idealised embankment on a layer of reinforcement over a soft foundation is presented. Effects of stiffness and strength of the reinforcement and of foundation depth on deformation behaviour of the embankment are studied. Tensile strain is induced in the reinforcement by both the embankment and the foundation. The benefit is not linearly proportional to reinforcement stiffness, but tends to a limiting value. Benefit is ultimately limited by tensile failure of the reinforcement or soil slip on the undersurface. Reinforcement is less effective as relative depth of foundation soil increases.

916402**Probabilistic safety evaluation of earth dams**

Knoesen, J S

Proc 11th International Conference on Soil Mechanics and Foundation Engineering, Rio de Janeiro, 13-18 August 1989 V2, P837-843. Publ Rotterdam: A A Balkema, 1989

A probabilistic approach to evaluate safety of existing dams against overtopping, piping erosion, and slope instability is presented. Probability of overtopping failure is determined by expressing demands in terms of erosion potential of the overtopping hydrograph against erosion resistance of the

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